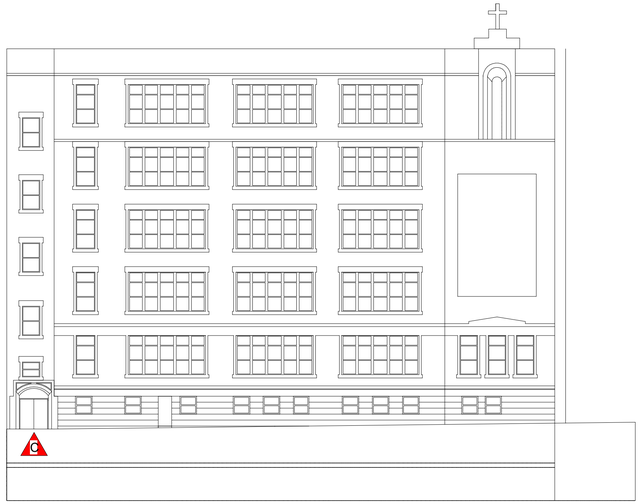
BEDFORD STUYVESANT NEW BEGINNINGS CHARTER SCHOOL

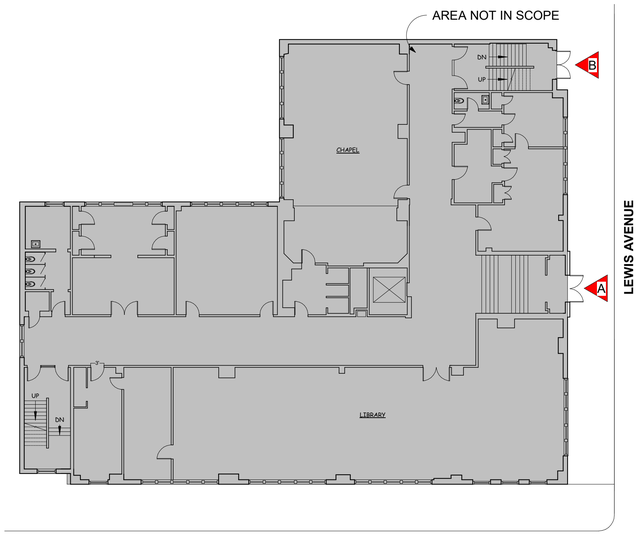
SWIMMING POOL INFILL BID DOCUMENTS

SHEET LIST

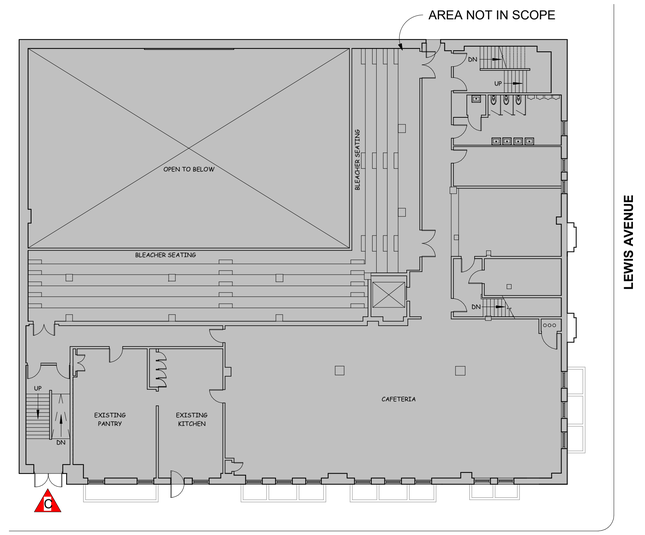
1 EAST ELEVATION
1/16" = 1'-0"



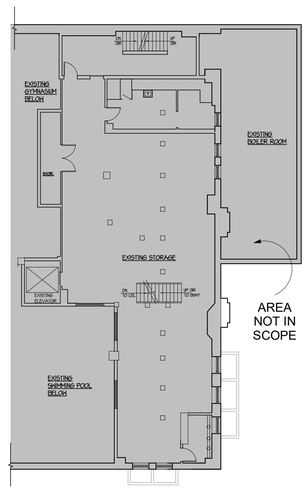
2 SOUTH ELEVATION
1/16" = 1'-0"



3 FIRST FLOOR PLAN
1/16" = 1'-0"



4 BASEMENT PLAN
1/16" = 1'-0"



5 CELLAR MEZZANINE PLAN
1/16" = 1'-0"

New Beginnings Charter School
82 Lewis Avenue, Brooklyn, NY 11206

Kliment Halsband Architects
322 Eighth Avenue, New York, NY 10001

CONSULTANTS:

Robert Silman Associates
32 Old Slip, 10th Floor, New York, NY 10003

Design 2147 LTD
52 Diamond Street, Brooklyn, NY 11222

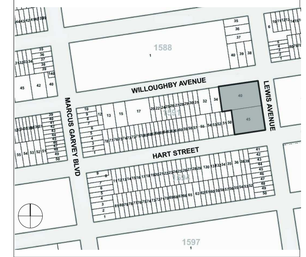
ISSUE:

BID SET

NOT FOR CONSTRUCTION

No.	Description	Date

PLOT PLAN:



PROJECT:
New Beginnings Charter School

82 Lewis Avenue, Brooklyn, NY 11206

DRAWING TITLE:
TITLE SHEET, REFERENCE PLANS AND ELEVATIONS

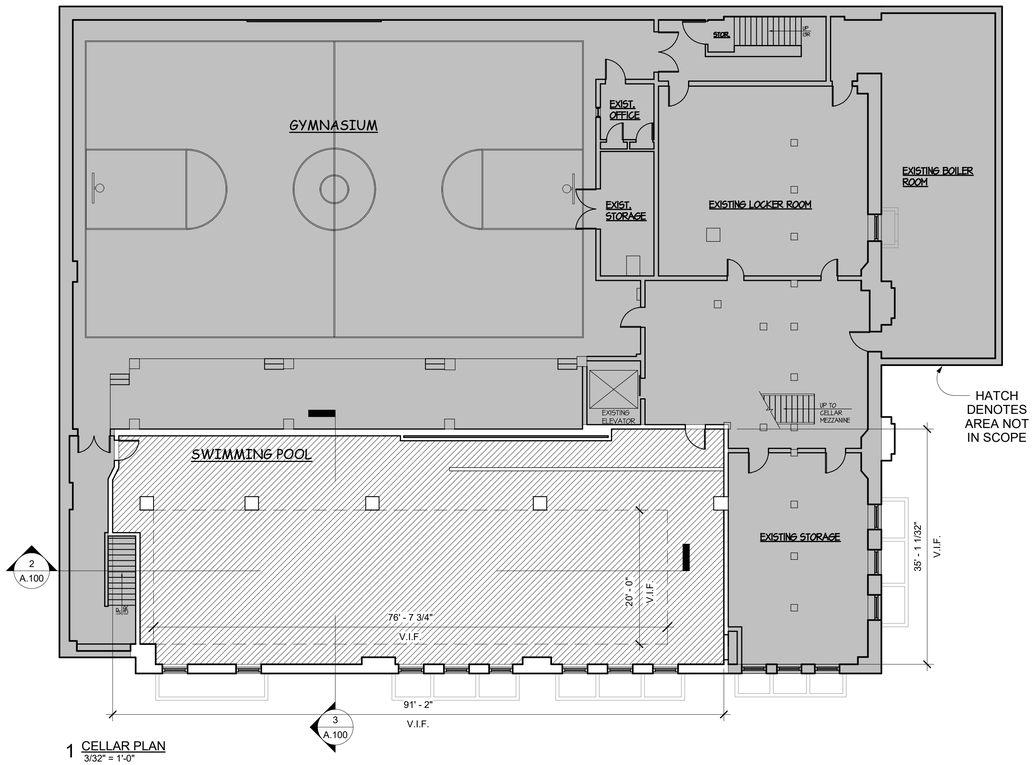
DRAWING NO:
A.001

SCALE:
1/16" = 1'-0"

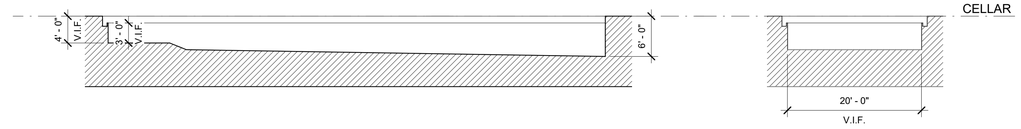
DATE:
22 JULY 2020

1907.1

NO: 1 OF: 4



1 CELLAR PLAN
3/32" = 1'-0"



2 SWIMMING POOL SECTION A
3/32" = 1'-0"

3 SWIMMING POOL SECTION B
3/32" = 1'-0"

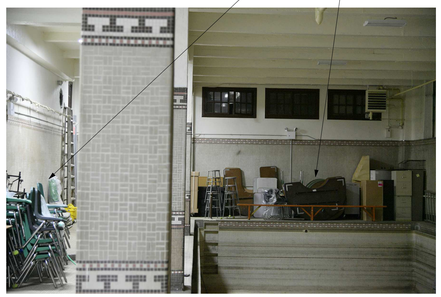
FINISH SCHEDULE												
ROOM NAME	FLOOR AREA	FLOOR FINISH	NORTH BASE	NORTH WALL	EAST BASE	EAST WALL	SOUTH BASE	SOUTH WALL	WEST BASE	WEST WALL	CLG FINISH	REMARKS
CELLAR												
SWIMMING POOL	2,995 SF	VCT	RB	ETR	RB	ETR	RB	ETR	RB	ETR	ETR	SEE GENERAL NOTES BELOW

GENERAL NOTES

1. PROVIDE CEMENT LEVELING COMPOUND AS NEEDED OVER THE EXISTING CERAMIC TILE FLOOR BEFORE INSTALLING THE VINYL COMPOSITE TILE (VCT). BASIS OF DESIGN IS "K-15" BY ARDEX OR APPROVED EQUAL. REMOVE CERAMIC TILE BULLNOSE EDGING AS REQUIRED FOR NEW STRUCTURAL FLOORING INFILL. PROVIDE A FLUSH TRANSITION FROM EXISTING FLOORING TO NEW FLOOR INFILL. LEVELING COMPOUND SHALL BE MIXED FOLLOWING MANUFACTURER'S INSTRUCTIONS. DENSITY: 115LB/CU FT; COMPRESSIVE STRENGTH: 4,000 PSI MINIMUM PER ASTM C 109; FOLLOW ASTM E 286 FOR FIRE/SMOKE RATING.
2. PROVIDE 12"X12" VCT FLOORING BY ARMSTRONG EQUIVALENT TO "JUBILEE WHITE" #52514 OF THE STANDARD EXCELON LINE. ARCHITECT WILL ENTERTAIN OTHER SIMILAR VCT OPTIONS TO MEET THE DEADLINE. CONTRACTOR TO PROVIDE ALTERNATE VCT FOR ARCHITECT APPROVAL.
3. PROVIDE 4" RUBBER COVE BASE (RB) WITH TOE, TYPE TS (THERMOSET), BY ROPPE EQUIVALENT TO PINNACLE "SLATE" #175. ARCHITECT WILL ENTERTAIN OTHER SIMILAR RB OPTIONS TO MEET THE DEADLINE. CONTRACTOR TO PROVIDE ALTERNATE RB FOR ARCHITECT APPROVAL.
4. CLEAN EXISTING CERAMIC TILE ON WALLS AND COLUMNS.
5. ASSUME 25% FLOOR AREA OF PATCHING EXISTING TILE AS REQUIRED.
6. IN ADDITION TO REMOVALS SPECIFICALLY INDICATED HEREIN, REMOVE EXISTING MATERIALS AS REQUIRED FOR INSTALLATION OF NEW WORK.

A POOL FACING WEST
12" = 1'-0"

OWNER RESPONSIBLE FOR REMOVING FURNITURE STORED IN POOL AREA PRIOR TO START OF CONSTRUCTION.



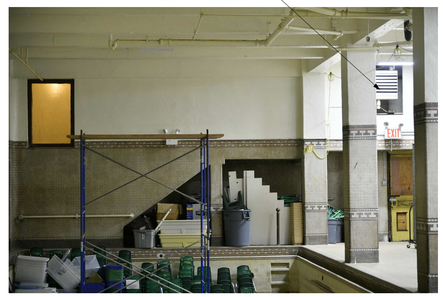
B POOL FACING EAST WALL
12" = 1'-0"

C POOL FACING SOUTH WALL
12" = 1'-0"

REMOVE CT BULL NOSE EDGE; SEE GENERAL NOTES.

D POOL FACING NORTH CORNER
12" = 1'-0"

PROTECT EXISTING TILE AND EQUIPMENT TO REMAIN, TYP.



E POOL FACING WEST WALL
12" = 1'-0"

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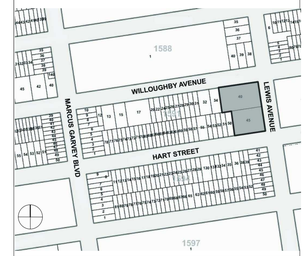
PLOT PLAN:



PROJECT:
New Beginnings Charter School

82 Lewis Avenue, Brooklyn, NY 11206

DRAWING TITLE:
CELLAR PLAN, FINISH SCHEDULE & NOTES

DRAWING NO:
A.100

SCALE:
As indicated

DATE:
22 JULY 2020

1907.1

NO: 2 OF: 4

GENERAL NOTES

1. ALL STRUCTURAL WORK SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND SHALL CONFORM TO THE PROJECT SPECIFICATIONS, INCLUDING THE NYC BUILDING CODE. ALL GOVERNING STANDARDS LISTED IN THESE NOTES SHALL BE THE EDITION REFERENCED IN THIS GOVERNING CODE.

2. CONTRACTOR SHALL PROVIDE TEMPORARY SHORING, BRACING AND SHEETING AND SHALL MAKE SAFE ALL FLOORS, ROOFS, WALLS AND ADJACENT PROPERTY AS PROJECT CONDITIONS REQUIRE. SHORING AND SHEETING SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE PROJECT JURISDICTION, HIRED BY THE CONTRACTOR, WHO SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR THE OWNER'S REVIEW.

3. DIMENSIONS AND ELEVATIONS OF EXISTING CONSTRUCTION GIVEN IN STRUCTURAL DRAWINGS ARE BASED ON INFORMATION CONTAINED IN VARIOUS ORIGINAL DESIGN AND CONSTRUCTION DOCUMENTS PROVIDED BY THE OWNER, AND LIMITED FIELD OBSERVATIONS AND MEASUREMENTS. THE CONTRACTOR SHALL VERIFY ALL INFORMATION PERTAINING TO EXISTING CONDITIONS BY ACTUAL MEASUREMENT AND OBSERVATION AT THE SITE. ALL DISCREPANCIES BETWEEN ACTUAL CONDITIONS AND THOSE SHOWN IN THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ENGINEER OF RECORD FOR EVALUATION BEFORE THE AFFECTED CONSTRUCTION IS PUT IN PLACE.

4. THE CONTRACT DRAWINGS AND SPECIFICATIONS ARE COMPLEMENTARY. THESE NOTES HIGHLIGHT RATHER THAN REPLACE THE SPECIFICATIONS CONTAINED IN THE PROJECT MANUAL. CONTRACTOR SHALL NOTIFY THE ARCHITECT AND ENGINEER OF ANY CONFLICTS FOR GUIDANCE.

CONCRETE

1. ALL CONCRETE WORK SHALL CONFORM TO THE FOLLOWING GOVERNING STANDARDS.
   - A. AMERICAN CONCRETE INSTITUTE (ACI) "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
   - B. (ACI 318).
   - C. ACI "MANUAL OF CONCRETE PRACTICE" LATEST EDITION
   - D. CONCRETE REINFORCING STEEL INSTITUTE (CRSI) "MANUAL OF STANDARD PRACTICE".

2. ALL CONCRETE COMPOSITE ON METAL DECK SHALL BE LIGHT WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS, UNLESS OTHERWISE NOTED.

3. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60 OR A775 EPOXY COATED WHEN CALLED OUT ON PLAN. REINFORCING STEEL SHALL BE DETAILED ACCORDING TO THE ACI "DETAILS AND DETAILING OF REINFORCEMENT", (ACI 315).

4. WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A1064, WITH A MINIMUM YIELD STRENGTH OF 65,000 PSI.

5. REINFORCING STEEL TO BE WELDED TO CONFORM TO ASTM A706 GRADE 60.

6. ALL GROUT SHALL BE NONSHRINK WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI.

10. SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. NO CONCRETE WORK SHALL COMMENCE WITHOUT APPROVED SHOP DRAWINGS.

11. CLEAN AND ROUGHEN TO ¼" AMPLITUDE ALL EXISTING CONCRETE SURFACES TO RECEIVE NEW CONCRETE PRIOR TO PLACEMENT.

12. WELDED WIRE REINFORCEMENT IN COMPOSITE CONSTRUCTION SHALL HAVE TENSION SPLICES AND BE ANCHORED AT DISCONTINUOUS EDGES.

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE FOLLOWING GOVERNING STANDARDS:
   - A. AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
   - B. AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
   - C. AMERICAN WELDING SOCIETY (AWS D1.1) "STRUCTURAL WELDING CODE – STEEL".
   - D. RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC) "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS".

2. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS:
   - A. WIDE FLANGE BEAMS, COLUMNS AND STRUCTURAL TEES: ASTM A992.
   - B. HOLLOW STRUCTURAL SECTIONS: ASTM A500, GRADE C.
   - C. STRUCTURAL PIPE SECTIONS: ASTM A53, GRADE B.
   - D. CHANNELS, ANGLES AND PLATES: ASTM A36 UNLESS OTHERWISE NOTED.
   - E. STRUCTURAL STEEL PLATE SHALL BE ASTM A572 GRADE 50 HAVING A MINIMUM YIELD POINT OF 50,000 PSI, EXCEPT WHERE THICKNESSES EXCEED 4" USE A572 GRADE 42 HAVING A MINIMUM YIELD POINT OF 42,000 PSI.
   - F. BOLTED CONNECTIONS SHALL BE PER ASTM F3125. GRADES ARE TO BE SELECTED AS FOLLOWS:
     - a. STANDARD BEAM TO BEAM/GIRDER: ASTM F3125, GRADES A325, F1852, A490 OR F2280 BOLTS IN SNUG-TIGHTENED JOINTS (⅞" DIAMETER MINIMUM WITH HARDENED WASHERS).
     - b. BEAM/GIRDER TO COLUMN CONNECTIONS, COLUMN SPLICES AND BOLTS EXPERIENCING TENSION LOADS (UNLESS OVERSIZED OR SLOTTED HOLES ARE USED, IN WHICH CASE SLIP-CRITICAL JOINTS SHALL BE USED): ASTM F3125, GRADES A325, F1852, A490 OR F2280 BOLTS IN PRETENSIONED JOINTS (⅞" DIAMETER MINIMUM WITH HARDENED WASHERS).
     - c. MOMENT CONNECTIONS AND BRACED FRAME CONNECTIONS: ASTM F3125, GRADES A325, F1852, A490 OR F2280 BOLTS IN SLIP CRITICAL JOINTS (⅞" DIAMETER MINIMUM WITH HARDENED WASHERS). FAYING SURFACES SHALL BE CLASS A UNLESS OTHERWISE NOTED.
     - d. FOR ASD: 3/4L: ALL BOLTS SHALL BE INSTALLED AS PRETENSIONED HIGH STRENGTH BOLTS AND MEET THE REQUIREMENTS FOR SURFACE PREPARATION FOR SLIP CRITICAL CONNECTIONS WITH CLASS A SLIP COEFFICIENT OR HIGHER. THE AVAILABLE SHEAR STRENGTH OF BOLTED JOINTS USING STANDARD HOLES SHALL BE CALCULATED AS THAT FOR BEARING TYPE JOINTS.

3. STEEL CONNECTION SHALL BE STANDARD AISC FRAMED BEAM CONNECTIONS, AND SHALL BE SELECTED OR COMPLETED BY AN EXPERIENCED STEEL DETAILER, UTILIZING LRFD LOADS AND PROCEDURES.

4. MINIMUM WELD SIZE IS ¼" FILLET UNLESS NOTED OTHERWISE.

5. ALL BEAMS EXCEPT CANTILEVER BEAMS SHALL BE FABRICATED AND INSTALLED WITH NATURAL CAMBER UP. CANTILEVER BEAMS SHALL BE FABRICATED AND INSTALLED SO THAT NATURAL CAMBER RAISES CANTILEVER END.

6. FIELD CUTTING OR BURNING OF STEEL IS PROHIBITED EXCEPT WITH THE EXPRESSED WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD (IN WHICH CASE ALL BURNING OF STEEL MUST CONFORM TO THE THERMAL CUTTING REQUIREMENTS OF AISC AND AWS).

7. WELDING SHALL BE PERFORMED BY CERTIFIED, AWS-QUALIFIED WELDERS. WELDING ELECTRODES FOR CARBON STEEL SHALL BE AWS 5.1, CLASS E70XX. FOR ASTM A572 GRADE 50 KSI PLATE USE ELECTRODE E7018 OR APPROVED EQUAL (OR ELECTRODES THAT MEET THE REQUIREMENT OF___). WELDING ELECTRODES FOR ASTM A276 STAINLESS STEEL, TYPE 304, SHALL CONFORM TO AWS A5.4 FOR SHIELDED METAL ARC WELDING, ELECTRODE CLASS E308; OR AWS A5.9 FOR GAS METAL ARC WELDING, ELECTRODE CLASS ER308. WELDING ELECTRODES FOR ASTM A276 TYPE 316L STAINLESS STEEL SHALL CONFORM TO AWS A5.4 FOR SHIELDED METAL ARC WELDING, ELECTRODE CLASS E316; OR AWS A5.9 FOR GAS METAL ARC WELDING, ELECTRODE CLASS ER316. WELDING ELECTRODES FOR JOINING STAINLESS STEEL TO CARBON STEEL SHALL CONFORM TO ELECTRODE CLASS E309/ER309.

11. SHOP AND ERECTION DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. NO FABRICATION OF STEEL SHALL COMMENCE WITHOUT APPROVED SHOP DRAWINGS.

STEEL DECKING

1. STEEL DECKING WORK SHALL CONFORM TO THE AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".

2. STEEL DECKING UNITS AND ACCESSORY ITEMS SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO ASTM A1008 OR A653 WITH A MINIMUM YIELD STRENGTH OF 33,000 PSI. BEFORE FORMING, THE STEEL SHEET SHALL RECEIVE A HOT DIP GALVANIZED COATING CONFORMING TO ASTM A653, GRADE 80.

3. STEEL DECKING SHALL BE SHORED AS REQUIRED BY PLANS OR BY SPAN AND LOAD CONDITIONS TO SUPPORT WET WEIGHT OF CONCRETE AND ALL CONSTRUCTION LOADS.

4. THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY APPROVED METHOD (TO BE SHOWN ON SHOP DRAWINGS) BETWEEN SUPPORTS, AT INTERVALS TO PROVIDE SUFFICIENT DIAPHRAGM STRENGTH TO MAINTAIN BUILDING ALIGNMENT AND TO SUSTAIN LOCAL CONSTRUCTION LOADS WITHOUT DISTORTION OR SEPARATION. MAXIMUM SPACING SHALL BE 3'-0" (2'-0" FOR NYC) BETWEEN SUPPORT BEAMS. END LAPS OF SHEETS SHALL BE A MINIMUM OF 2 INCHES.

5. EXCEPT AS OTHERWISE NOTED, DECK SHALL BE ATTACHED TO STRUCTURAL STEEL BY 3/4"† FUSION WELDS @12"O.C. AT END AND INTERIOR SUPPORTS PERPENDICULAR TO THE DECK SPAN AND AT EDGE AND INTERIOR SUPPORTS PARALLEL TO THE DECK SPAN. WELDS MAY BE OMITTED IN RIBS IN WHICH SHEAR CONNECTORS ARE TO BE APPLIED, EXCEPT THAT EACH DECK SECTION SHALL HAVE SUFFICIENT WELDS TO ADEQUATELY SECURE THE DECK, BRING THE DECK INTO DIRECT CONTACT WITH THE SUPPORTING STEEL AND TO PROVIDE SUFFICIENT DIAPHRAGM STRENGTH TO MAINTAIN BUILDING ALIGNMENT.

6. AS AN ALTERNATE TO PUDDLE WELDS FOR STEEL DECK ATTACHMENT TO STRUCTURAL STEEL, HILTI X-HSN-24 OR X-DNP-19 POWDER ACTUATED FASTENERS, OR AN APPROVED EQUAL, WITH EQUIVALENT OR GREATER CAPACITY TO THE SPECIFIED ATTACHMENT MAY BE USED. PRIOR TO INSTALLATION THE CONTRACTOR SHALL SUBMIT ALTERNATE FASTENING PATTERN TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL. SUBMITTAL SHALL INCLUDE PROPOSED ALTERNATE PATTERN AND ANY CALCULATIONS OR SUPPORTING MANUFACTURER DATA NEEDED TO DEMONSTRATE THAT THE PATTER MEETS OR EXCEEDS THE CAPACITY OF THE SPECIFIED ATTACHMENT.

7. POWDER ACTUATED FASTENERS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. CONTRACTOR SHALL BE CERTIFIED AND TRAINED BY THE MANUFACTURER'S REPRESENTATIVE ON PROPER USE PRIOR TO INSTALLATION.

8. PRIOR TO FABRICATION, THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR THE STEEL DECKING, SHOWING DECK GAUGE, SIZE AND LAYOUT AS WELL AS CLOSURE CONDITIONS, WELDS TO SUPPORTS AND SIDE LAP DETAILS.

9. ALL REINFORCED OPENINGS IN STEEL DECK SHALL BE INSTALLED BY STEEL DECK SUBCONTRACTOR. STEEL DECK SUBCONTRACTOR TO PROVIDE REINFORCING AS PER TYPICAL DETAILS.

10. AT STEEL DECK WITHOUT CONCRETE FILL THE FOLLOWING MAY BE ATTACHED WITHOUT SPECIFIC APPROVAL OF THE STRUCTURAL ENGINEER: ACOUSTICAL TILE AND GYPSUM BOARD CEILING ONLY; NO PIPING, DUCTING OR CONDUIT. MAXIMUM CEILING WEIGHT = 3.5 PSF. MAXIMUM WIRE HANGER LOAD = 60#.

11. WHERE SUSPENSION OF HANGER WIRES ARE REQUIRED BY OTHERS, VERIFY AND COORDINATE LOCATIONS, PATTERNS, SPACING, ETC. WITH THE APPROPRIATE TRADE. DRILL OR PUNCH HOLES AT BOTTOM OF DECK FLUTES OF SUFFICIENT SIZE TO PASS SUPPORT WIRES. WIRE SUPPORTS SHALL BE LOOPED AND SECURED WITH A MINIMUM OF THREE (3) TIGHT TURNS AROUND A MINIMUM 1½"x12" LONG FURRING CHANNEL OR NO. 3x12" LONG REINFORCING BAR CENTERED ABOVE THE HOLE AND LAID IN THE DECK FLUTES.

POST-INSTALLED ADHESIVE AND MECHANICAL ANCHORS

1. POST INSTALLED ANCHORAGE SHALL BE INSTALLED BY QUALIFIED PERSONNEL PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII), AS INCLUDED IN THE ANCHOR PACKAGING, TO INTACT BASE MATERIAL. INSTALLATION OF ANCHORS SHALL BE CARRIED OUT BY AN INSTALLER TRAINED TO INSTALL THE SPECIFIED ANCHORS. NOTIFY ENGINEER OF RECORD PRIOR TO INSTALLATION IF BASE MATERIAL CONDITION DEVIATES FROM STRUCTURAL DRAWINGS OR ASSUMPTIONS AND CONDITIONS OF THE MPII. ALL HOLES SHALL BE DRY AND HAMMER DRILLED UNLESS OTHERWISE NOTED, AND ALL CONCRETE BASE MATERIAL TO RECEIVE ADHESIVE ANCHORS SHALL HAVE A MINIMUM AGE OF 21 DAYS.

2. INSTALLATION OF ADHESIVE ANCHORS IN A HORIZONTAL OR UPWARDLY INCLINED ORIENTATION AND SUPPORTING A SUSTAINED TENSION LOAD SHALL BE PERFORMED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM OR APPROVED EQUAL. PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS, PROVIDE OWNER AND ENGINEER OF RECORD WITH DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL HORIZONTAL OR UPWARDLY INCLINED ADHESIVE ANCHORS SUPPORTING SUSTAINED TENSION LOADS ARE TRAINED AND CERTIFIED.

   A. OVERHEAD ADHESIVE ANCHORS MUST BE INSTALLED USING THE PISTON PLUG SYSTEM SPECIFIED BY THE MPII AND PRODUCED BY THE CORRESPONDING MANUFACTURER FOR THE ANCHOR SYSTEM BEING INSTALLED.

3. EXISTING REINFORCING BARS IN THE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. REINFORCING BARS SHALL NOT BE CUT WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER OF RECORD. UNLESS NOTED ON THE DRAWINGS THAT THE EXISTING REBARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS BY A MEANS APPROVED BY THE ENGINEER OF RECORD.

4. ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS, PROXIMITY OF ANCHORS TO EDGE OF CONCRETE, AND EMBEDMENT DEPTH INTO THE SUBSTRATE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING, EDGE CLEARANCES, AND EMBEDMENT DEPTHS INDICATED ON THE DRAWINGS.

5. UNLESS OTHERWISE INDICATED, POST INSTALLED ANCHORAGE SHALL BE ADHESIVE TYPE HILTI HIT-HY 200-R INTO CONCRETE OR HILTI HIT-HY 270 INTO BRICK MASONRY. GROUT FILLED CMU OR UNGROUTED CMU BASE MATERIAL. PROVIDE MESH SCREEN IN UNGROUTED CMU, UNREINFORCED MASONRY CONSTRUCTION, AND BRICK MASONRY WITH HOLES OR VOIDS.

6. SUBSTITUTION REQUESTS FOR ALTERNATE ANCHORAGE PRODUCTS SHALL BE SUBMITTED TO ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO USE. THIS SHALL INCLUDE MANUFACTURER PRODUCT DATA AND CALCULATIONS DEMONSTRATING THAT THE PROPOSED SUBSTITUTE CAN ACHIEVE THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY THE MANUFACTURER OR SUCH OTHER METHOD AS APPROVED BY THE ENGINEER OF RECORD. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC-ES EVALUATION REPORT SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE, SEISMIC USE, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF MPII. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE AND MUST PROVIDE INFORMATION ON THESE ITEMS. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE ENGINEER OF RECORD PRIOR TO USE.

SPECIAL INSPECTIONS

1. SPECIAL INSPECTIONS REQUIRED BY THE LOCAL JURISDICTION SHALL BE PERFORMED BY A TESTING AGENCY PROVIDED BY THE OWNER FOR THE FOLLOWING ITEMS:
   - A. STRUCTURAL STEEL – WELDING (BC 1704.3.1)
   - B. STRUCTURAL STEEL – DETAILS (BC 1704.3.2)
   - C. STRUCTURAL STEEL – HIGH STRENGTH BOLTING (BC 1704.3.3)
   - D. STRUCTURAL COLD-FORMED STEEL (BC 1704.3.4)
   - E. CONCRETE – CAST-IN-PLACE (BC 1704.4)
   - F. POST-INSTALLED ANCHORS (BBP 2014-018, 2014-019) (BC 1704.32)

THE TESTING AGENCY FOR THE INSPECTIONS SHALL FILE ALL APPROPRIATE FORMS WITH THE BUILDING DEPARTMENT.

THE WORK MEETS THE EXEMPTION CRITERIA OF BUILDINGS BULLETIN 2009-026, ITEM IV, AND I HEREBY ELECT TO WAIVE THE REQUIREMENT OF CONCRETE TESTING AND OF THE TR-2 AND/OR TR-3 FORM.

- THE TOTAL STRUCTURAL CONCRETE SPECIFIED FOR THE PROJECT IS LESS THAN 50 CUBIC YARDS;
- THE STRUCTURAL DESIGN OF THE CONCRETE BASED ON SPECIFIED COMPRESSIVE STRENGTH, f'c, NO GREATER THAN 2,500 POUNDS PER SQUARE IN (PSI), REGARDLESS OF THE COMPRESSIVE STRENGTH SPECIFIED IN THE CONSTRUCTION DOCUMENTS OR USED IN THE CONSTRUCTION;
- THE CONCRETE TO BE PLACED IS SPECIFIED TO HAVE A COMPRESSIVE STRENGTH OF AT LEAST 4,000 PSI.

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IT IS A VIOLATION OF THE STATE EDUCATION LAW SECTION 7209 (2) FOR ANY PERSON TO ALTER AN ITEM IN ANY WAY UNLESS SUCH PERSON IS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, AND THE ENGINEER STAMPS SUCH CHANGES

No.	Description	Date

PLOT PLAN:

PROJECT:
New Beginnings Charter School

82 Lewis Avenue, Brooklyn, NY 11206

DRAWING TITLE:
GENERAL NOTES

DRAWING NO:
S.01

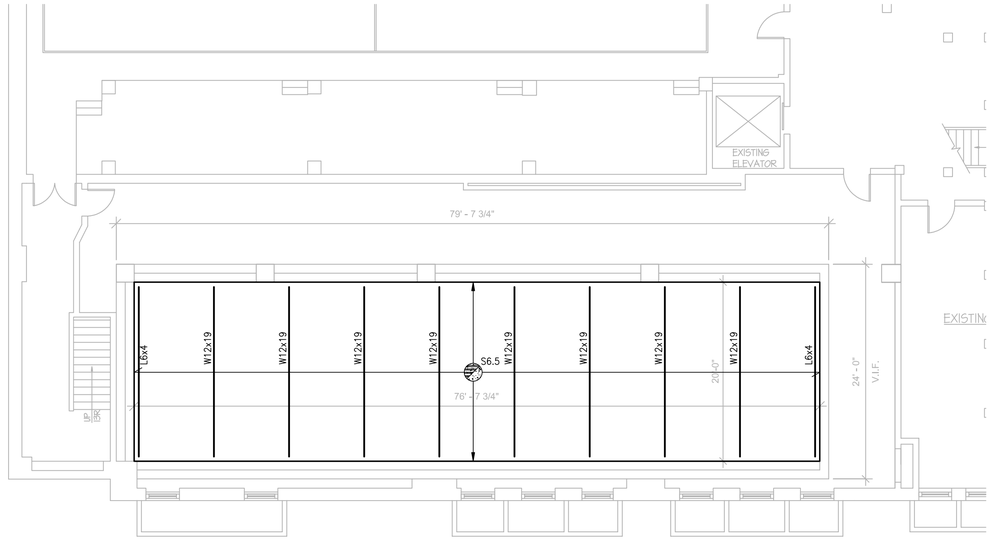
SCALE:
AS SHOWN

DATE:
22 JULY 2020

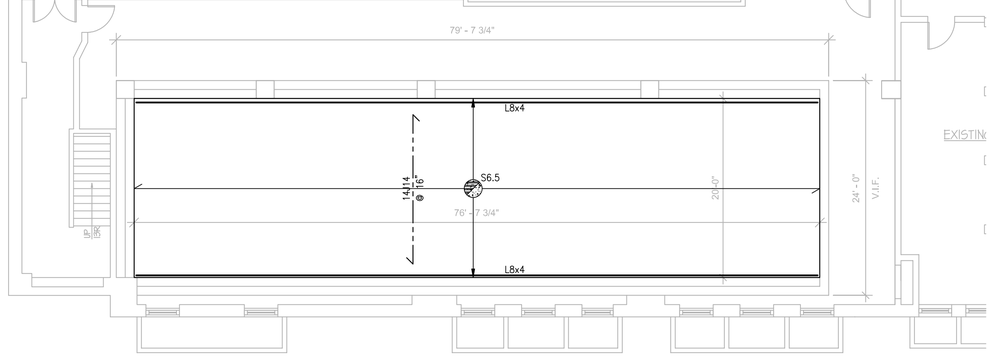
1907.1

NO: 3 OF: 4

SILMAN NO. ####



1 S-.101 SWIMMING POOL INFIL PLAN
SCALE: 1/8"=1'-0"



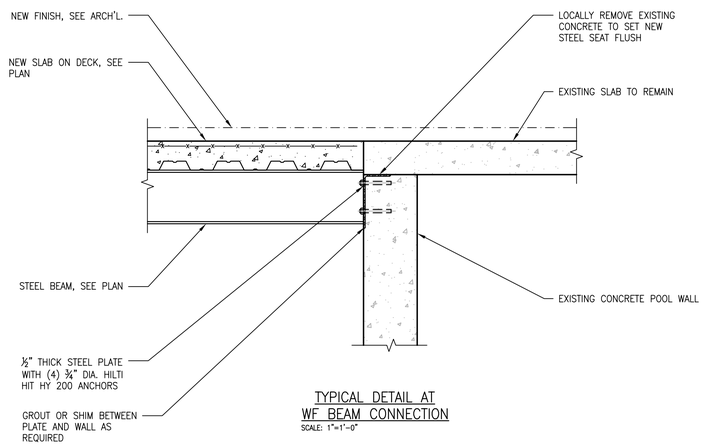
2 S-.101 SWIMMING POOL INFIL PLAN–ALTERNATE
SCALE: 1/8"=1'-0"

LEGEND:

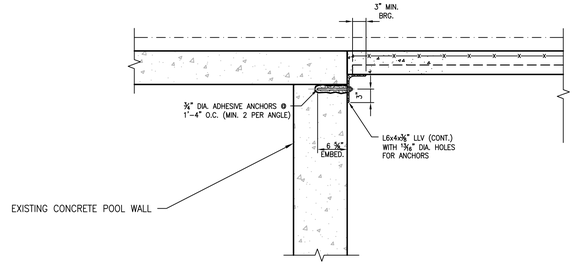
NEW LIGHT GAUGE JOIST

NEW STEEL BEAM

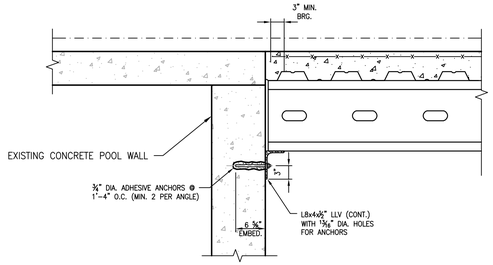
S6.5 — 4½" L.W. CONCRETE ON 2"–18 GAUGE METAL DECK (6½" TOTAL THICKNESS)



TYPICAL DETAIL AT WF BEAM CONNECTION
SCALE: 1"=1'-0"



TYPICAL DETAIL SHELF ANGLE AT SIDE WALL FOR COMPOSITE FLOOR DECK SUPPORT
SCALE: 1"=1'-0"



TYPICAL DETAIL AT LIGHT GAUAGE JOIST SUPPORT
SCALE: 1"=1'-0"

New Beginnings Charter School
82 Lewis Avenue, Brooklyn, NY 11206

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ISSUE:

BID SET

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No.	Description	Date

PLOT PLAN:

PROJECT:
New Beginnings Charter School

82 Lewis Avenue, Brooklyn, NY 11206

DRAWING TITLE:
SWIMMING POOL PART PLANS

DRAWING NO: **S.101**

SCALE: AS SHOWN

DATE: 22 JULY 2020

1907.1

NO: 4 OF: 4

SILMAN NO. ####